

# Project Details

Design Code: AS 4100-1998  
Country: Australia



User Name: Sam  
Project Name: Australian Project  
Project ID: Sydney1234  
Company: XYZ Pty Ltd  
Designer: Sam Carigliano  
Client: ABC Pty Ltd  
Unit System: metric

NOTE: The calculations for this design code are in **BETA** stage development. If you notice any irregularities or problems please contact support@skyciv.com.

## Design Input Information

Design Factors	
$\phi$	0.9

Design Materials			
ID	E (MPa)	Fy (MPa)	Fu (MPa)
1	200000	260	410

Section Dimensions								
ID	Name	d (m)	tw (m)	bt (m)	bb (m)	tt (m)	tb (m)	r (m)
1	360 UB 57	3.586e-1	7.950e-3	1.721e-1	1.721e-1	1.300e-2	1.300e-2	1.020e-2
2	310 UC 118	3.145e-1	1.190e-2	3.068e-1	3.068e-1	1.870e-2	1.870e-2	1.520e-2

Section Properties								
ID	Name	A (m <sup>2</sup> )	J (m <sup>4</sup> )	I <sub>yp</sub> (m <sup>4</sup> )	I <sub>zp</sub> (m <sup>4</sup> )	I <sub>w</sub> (m <sup>6</sup> )	S <sub>yp</sub> (m <sup>3</sup> )	S <sub>zp</sub> (m <sup>3</sup> )
1	360 UB 57	7.2200e-3	3.3100e-7	1.1100e-5	1.6100e-4	3.3100e-7	1.9900e-4	1.0100e-3
2	310 UC 118	1.5000e-2	1.6000e-6	9.0100e-5	2.7600e-4	1.9700e-6	8.9200e-4	1.9500e-3

Member Properties											
Member ID	Section ID	KzL (m)	KyL (m)	Lb (m)	$\alpha_m$	k <sub>t</sub>	k <sub>l</sub>	k <sub>r</sub>	LST	LSC	
1	1	1.00	1.00	1	1	1	1	1	300	200	
2	2	2.00	2.00	1	1	1	1	1	300	200	
3	2	2.00	2.00	1	1	1	1	1	300	200	
4	1	1.00	1.00	1	1	1	1	1	300	200	

## Member Design Capacity

Member ID	$\phi N_t$ (kN)	$\phi N_s$ (kN)	$\phi M_{sz}$ (kN-m)	$\phi M_{sy}$ (kN-m)	$\phi V_{vy}$ (kN)	$\phi V_{vz}$ (kN)	$\phi N_c$ (kN)	$\phi M_b$ (kN-m)
1	1689.48	1689.48	236.34	45.28	400.26	628.23	1617.77	236.15
2	3510.00	3510.00	456.30	206.16	525.45	1610.99	3357.23	469.03
3	3510.00	3510.00	456.30	206.16	525.45	1610.99	3357.23	469.03
4	1689.48	1689.48	236.34	45.28	400.26	628.23	1617.77	236.15

## Design Ratio

Member ID	$N^* / \phi N_s$	$M^*_z / \phi M_{sz}$	$M^*_y / \phi M_{sy}$	$V^*_y / \phi V_{vy}$	$V^*_z / \phi V_{vz}$	Combined Strength	$N^* / \phi N_c$	$M^*_z / \phi M_b$	Combined Buckling	KL / r	Status
1	0.02	0.04	0.22	0.05	0.03	0.28	0.02	0.04	0.14	0.13	OK
2	0.01	0.02	0.05	0.04	0.01	0.08	0.01	0.02	0.02	0.13	OK
3	0.01	0.02	0.05	0.04	0.01	0.08	0.01	0.02	0.02	0.13	OK
4	0.02	0.04	0.22	0.05	0.03	0.28	0.02	0.04	0.14	0.13	OK

# Definitions

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$\phi$	Capacity factors in table 3.4
E	Modulus of elasticity
F <sub>y</sub>	Specified minimum yield stress
F <sub>u</sub>	Specified minimum tensile strength
A	Cross-sectional area
J	Torsional constant
I <sub>yp</sub>	Moment of inertia about the Y axes
I <sub>zp</sub>	Moment of inertia about the Z axes
I <sub>w</sub>	Warping constant
S <sub>yp</sub>	Plastic section modulus about the Y axis
S <sub>zp</sub>	Plastic section modulus about the Z axis
KL	Effective length
$\alpha_m$	Moment modification factor
k <sub>t</sub>	Twist restraint factor given in Table 5.6.3(1)
k <sub>l</sub>	Load height factor given in Table 5.6.3(2)
k <sub>r</sub>	Lateral rotation restraint factor given in Table 5.6.3(3)
L <sub>b</sub>	Length between braced points
LST	Limited slenderness for tension
LSC	Limited slenderness for compression
LD	Limited deflection
N <sub>t</sub>	The nominal section capacity in tension determined in accordance with Clause 7.2
N <sub>s</sub>	The nominal section capacity determined in accordance with Clause 6.2 (compression)
M <sub>sz,Rd</sub>	The nominal section moment capacity, as specified in Clause 5.2, for bending about the z-axis
M <sub>sy</sub>	the nominal section moment capacity, as specified in Clause 5.2, for bending about the y-axis
V <sub>vy</sub>	the nominal shear capacity of the web determined from either Clause 5.11.2 or Clause 5.11.3 (along Y axis)
V <sub>vz</sub>	the nominal shear capacity of the web determined from either Clause 5.11.2 or Clause 5.11.3 (along Z axis)
N <sub>c</sub>	The nominal member capacity determined in accordance with Clause 6.3 (compression)
M <sub>b</sub>	The nominal member moment capacity, as specified in Clause 5.3 or 5.6, for bending about the z-axis
N <sub>ed</sub> / N <sub>Rd</sub>	Design ratio in case of axial force
M <sup>*</sup> <sub>z</sub> / $\phi$ M <sub>sz</sub>	Design ratio in case of bending about Z axis
M <sup>*</sup> <sub>y</sub> / $\phi$ M <sub>sy</sub>	Design ratio in case of bending about Y axis
V <sup>*</sup> <sub>y</sub> / $\phi$ V <sub>vy</sub>	Design ratio in case of shear along Y axis
V <sup>*</sup> <sub>z</sub> / $\phi$ V <sub>vz</sub>	Design ratio in case of shear along Z axis
N <sup>*</sup> / $\phi$ N <sub>c</sub>	Design ratio in case of buckling (compression)
M <sup>*</sup> <sub>z</sub> / $\phi$ M <sub>b</sub>	Design ratio in case of buckling (bending)
KL/r	Design ratio in case of section slenderness
$\delta$	Design ratio in case of member deflection
OK	Capacity is provided
NG	Capacity is not provided